SEISMIC HAZARD ZONE REPORT FOR THE LOVEJOY BUTTES 7.5-MINUTE QUADRANGLE, LOS ANGELES COUNTY, CALIFORNIA

2004

DEPARTMENT OF CONSERVATION
California Geological Survey

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SEISMIC HAZARD ZONE REPORT 086

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ILLUSTRATIONS

EXECUTIVE SUMMARY

This report summarizes the methods and sources of information used to prepare the Seismic Hazard Zone Map for the Lovejoy Buttes 7.5-Minute Quadrangle, Los Angeles County, California. The map displays the boundaries of zones of required investigation for liquefaction and earthquake-induced landslides over an area of approximately 62 square miles at a scale of 1 inch = 2,000 feet.

The Lovejoy Buttes Quadrangle is in the Antelope Valley in northeastern Los Angeles County. The center of the area is about 18 miles east of Palmdale and 45 miles northeast of the Los Angeles Civic Center. The area is characterized by typical high desert scrubland and grassland of low local relief except for the cluster of hills in the northwestern quarter called Lovejoy Buttes and three small hills in the northeastern corner. Several braided channels of intermittent streams that collectively belong to Big Rock Wash cut across the western part of the quadrangle. No settlements exist within the quadrangle, which consists entirely of unincorporated Los Angeles County land. One of the peaks of Lovejoy Buttes reaches 3251 feet in elevation. The highest point, above 3380 feet, in the quadrangle, however, is in the southeastern corner. The lowest point, below 2610 feet, is north of the buttes on the northern boundary. Access to the region is primarily via State Highway 138 (Pearblossom Highway) and east-west avenues (lettered) and north-south streets (numbered).

The map is prepared by employing geographic information system (GIS) technology, which allows the manipulation of three-dimensional data. Information considered includes topography, surface and subsurface geology, borehole data, historical ground-water levels, existing landslide features, slope gradient, rock-strength measurements, geologic structure, and probabilistic earthquake shaking estimates. The shaking inputs are based upon probabilistic seismic hazard maps that depict peak ground acceleration, mode magnitude, and mode distance with a 10 percent probability of exceedance in 50 years.

The liquefaction zone covers a broad region in the western half of the Lovejoy Buttes Quadrangle in the vicinity of Big Rock Wash and into the central part of the quadrangle where historically highest ground water depth is 40 feet or less. Steep slopes in crystalline bedrock occur only within Lovejoy Buttes and other small hills in this mostly alluviated quadrangle. The earthquake-induced landslide zone covers only about one percent of the quadrangle.

How to view or obtain the map

Seismic Hazard Zone Maps, Seismic Hazard Zone Reports and additional information on seismic hazard zone mapping in California are available on the California Geological Survey's Internet page: http://www.conservation.ca.gov/CGS/index.htm

Paper copies of Official Seismic Hazard Zone Maps, released by CGS, which depict zones of required investigation for liquefaction and/or earthquake-induced landslides, are available for purchase from:

BPS Reprographic Services
945 Bryant Street
San Francisco, California 94105
(415) 512-6550

Seismic Hazard Zone Reports (SHZR) summarize the development of the hazard zone map for each area and contain background documentation for use by site investigators and local government reviewers. These reports are available for reference at CGS offices in Sacramento, San Francisco, and Los Angeles. **NOTE: The reports are not available through BPS Reprographic Services.**

INTRODUCTION

The Seismic Hazards Mapping Act (the Act) of 1990 (Public Resources Code, Chapter 7.8, Division 2) directs the California Department of Conservation (DOC), Division of Mines and Geology (DMG) [now called California Geological Survey (CGS)] to delineate seismic hazard zones. The purpose of the Act is to reduce the threat to public health and safety and to minimize the loss of life and property by identifying and mitigating seismic hazards. Cities, counties, and state agencies are directed to use the seismic hazard zone maps in their land-use planning and permitting processes. They must withhold development permits for a site within a zone until the geologic and soil conditions of the project site are investigated and appropriate mitigation measures, if any, are incorporated into development plans. The Act also requires sellers (and their agents) of real property within a mapped hazard zone to disclose at the time of sale that the property lies within such a zone. Evaluation and mitigation of seismic hazards are to be conducted under guidelines adopted by the California State Mining and Geology Board (SMGB) (DOC, 1997). The text of this report is on the Internet at http://gmw.consrv.ca.gov/shmp/webdocs/sp117.pdf

The Act also directs SMGB to appoint and consult with the Seismic Hazards Mapping Act Advisory Committee (SHMAAC) in developing criteria for the preparation of the seismic hazard zone maps. SHMAAC consists of geologists, seismologists, civil and structural engineers, representatives of city and county governments, the state insurance commissioner and the insurance industry. In 1991 SMGB adopted initial criteria for delineating seismic hazard zones to promote uniform and effective statewide implementation of the Act. These initial criteria provide detailed standards for mapping regional liquefaction hazards. They also directed CGS to develop a set of probabilistic seismic maps for California and to research methods that might be appropriate for mapping earthquake-induced landslide hazards.

In 1996, working groups established by SHMAAC reviewed the prototype maps and the techniques used to create them. The reviews resulted in recommendations that 1) the process for zoning liquefaction hazards remain unchanged and 2) earthquake-induced landslide zones be delineated using a modified Newmark analysis.

This Seismic Hazard Zone Report summarizes the development of the hazard zone map. The process of zoning for liquefaction uses a combination of Quaternary geologic mapping, historical ground-water information, and subsurface geotechnical data. The process for zoning earthquake-induced landslides incorporates earthquake loading, existing landslide features, slope gradient, rock strength, and geologic structure. Probabilistic seismic hazard maps, which are the underpinning for delineating seismic hazard zones, have been prepared for peak ground acceleration, mode magnitude, and mode distance with a 10 percent probability of exceedance in 50 years (Petersen and others, 1996) in accordance with the mapping criteria.

This report summarizes seismic hazard zone mapping for potentially liquefiable soils and earthquake-induced landslides in the Lovejoy Buttes 7.5-Minute Quadrangle.

SECTION 1
LIQUEFACTION EVALUATION REPORT

Liquefaction Zones in the Lovejoy Buttes 7.5-Minute Quadrangle, Los Angeles County, California

By
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California Department of Conservation
California Geological Survey

PURPOSE

The Seismic Hazards Mapping Act (the Act) of 1990 (Public Resources Code, Chapter 7.8, Division 2) directs the California Department of Conservation (DOC), Division of Mines and Geology (DMG) [now called California Geological Survey (CGS)] to delineate Seismic Hazard Zones. The purpose of the Act is to reduce the threat to public health and safety and to minimize the loss of life and property by identifying and mitigating seismic hazards. Cities, counties, and state agencies are directed to use seismic hazard zone maps developed by CGS in their land-use planning and permitting processes. The Act requires that site-specific geotechnical investigations be performed prior to permitting most urban development projects within seismic hazard zones. Evaluation and mitigation of seismic hazards are to be conducted under guidelines adopted by the California State Mining and Geology Board (SMGB) (DOC, 1997). The text of this report is on the Internet at http://gmw.consrv.ca.gov/shmp/webdocs/sp117.pdf

Following the release of DMG Special Publication 117 (DOC, 1997), agencies in the Los Angeles metropolitan region sought more definitive guidance in the review of geotechnical investigations addressing liquefaction hazards. The agencies made their request through the Geotechnical Engineering Group of the Los Angeles Section of the American Society of Civil Engineers (ASCE). This group convened an implementation committee under the auspices of the Southern California Earthquake Center (SCEC).

The committee, which consisted of practicing geotechnical engineers and engineering geologists, released an overview of the practice of liquefaction analysis, evaluation, and mitigation techniques (SCEC, 1999). This text is also on the Internet at: http://www.scec.org/

This section of the evaluation report summarizes seismic hazard zone mapping for potentially liquefiable soils in the Lovejoy Buttes 7.5-Minute Quadrangle. Section 2 (addressing earthquake-induced landslides) and Section 3 (addressing potential ground shaking), complete the report, which is one of a series that summarizes production of similar seismic hazard zone maps within the state (Smith, 1996). Additional information on seismic hazards zone mapping in California is on CGS's Internet web page: http://www.conservation.ca.gov/CGS/index.htm

BACKGROUND

Liquefaction-induced ground failure historically has been a major cause of earthquake damage in southern California. During the 1971 San Fernando and 1994 Northridge earthquakes, significant damage to roads, utility pipelines, buildings, and other structures in the Los Angeles area was caused by liquefaction-induced ground displacement.

Localities most susceptible to liquefaction-induced damage are underlain by loose, water-saturated, granular sediment within 40 feet of the ground surface. These geological and ground-water conditions exist in parts of southern California, most notably in some densely populated valley regions and alluviated floodplains. In addition, the potential for strong earthquake ground shaking is high because of the many nearby active faults. The combination of these factors constitutes a significant seismic hazard in the southern California region in general, including areas in the Lovejoy Buttes Quadrangle.

METHODS SUMMARY

Characterization of liquefaction hazard presented in this report requires preparation of maps that delineate areas underlain by potentially liquefiable sediment. The following were collected or generated for this evaluation:

- Existing geologic maps were used to provide an accurate representation of the spatial distribution of Quaternary deposits in the study area. Geologic units that generally are susceptible to liquefaction include late Quaternary alluvial and fluvial sedimentary deposits and artificial fill

- Construction of shallow ground-water maps showing the historically highest known ground-water levels

- Quantitative analysis of geotechnical data to evaluate liquefaction potential of deposits

- Information on potential ground shaking intensity based on CGS probabilistic shaking maps

The data collected for this evaluation were processed into a series of geographic information system (GIS) layers using commercially available software. The liquefaction zone map was derived from a synthesis of these data and according to criteria adopted by the SMGB (DOC, 2000).

SCOPE AND LIMITATIONS

Evaluation for potentially liquefiable soils generally is confined to areas covered by Quaternary (less than about 1.6 million years) sedimentary deposits. Such areas within the Lovejoy Buttes Quadrangle consist mainly of alluvial fan and modern wash areas. CGS's liquefaction hazard evaluations are based on information on earthquake ground shaking, surface and subsurface lithology, geotechnical soil properties, and ground-water depth, which is gathered from various sources. Although selection of data used in this evaluation was rigorous, the quality of the data used varies. The State of California and the Department of Conservation make no representations or warranties regarding the accuracy of the data obtained from outside sources.

Liquefaction zone maps are intended to prompt more detailed, site-specific geotechnical investigations, as required by the Act. As such, liquefaction zone maps identify areas where the potential for liquefaction is relatively high. They do not predict the amount or direction of liquefaction-related ground displacements, or the amount of damage to facilities that may result from liquefaction. Factors that control liquefaction-induced ground failure are the extent, depth, density, and thickness of liquefiable materials, depth to ground water, rate of drainage, slope gradient, proximity to free faces, and intensity and duration of ground shaking. These factors must be evaluated on a site-specific basis to assess the potential for ground failure at any given project site.

Information developed in the study is presented in two parts: physiographic, geologic, and hydrologic conditions in PART I, and liquefaction and zoning evaluations in PART II.

PART I

PHYSIOGRAPHY

Study Area Location and Physiography

The Lovejoy Buttes 7.5-Minute Quadrangle covers approximately 62 square miles in the Antelope Valley in northeastern Los Angeles County. It includes part of the city of Lake Los Angeles, the settlement of Llano, and scattered farming residences within unincorporated Los Angeles County land. The center of the area is about 18 miles east of

Palmdale and 45 miles northeast of the Los Angeles Civic Center. Topographically, the area is characterized by typical high desert scrubland and grassland of low local relief except for the cluster of hills in the northwestern quarter called Lovejoy Buttes and three small hills in the northeastern corner. Several braided channels of intermittent streams that collectively belong to Big Rock Wash cut across the western part of the quadrangle. One of the peaks of Lovejoy Buttes reaches 3,251 feet in elevation. The highest point, above 3,380 feet, in the quadrangle, however, is in the southeastern corner on the alluvial apron that slopes northward from Holcomb Ridge in the adjacent Valyermo Quadrangle. The lowest point, below 2,610 feet, is north of the buttes on the northern boundary. Access to the region is primarily via State Highway 138 (Pearblossom Highway), Palmdale Boulevard, and 170th Street East. Numerous east-west avenues (lettered) and north-south streets (numbered) provide local access.

GEOLOGY

Bedrock and Surficial Geology

Geologic units that are generally susceptible to liquefaction include late Quaternary alluvial and fluvial sedimentary deposits and artificial fill. For this evaluation, the Quaternary geologic map of the eastern Antelope Valley (Ponti and Burke, 1980, scale 1:62,500) was digitized by the Southern California Areal Mapping Project. The geology for the Lovejoy Buttes Quadrangle was extracted from this regional map, with minor modifications added by CGS to form a 1:24,000-scale map. Plate 1.1 shows the generalized Quaternary geology of the Lovejoy Buttes Quadrangle that was used in combination with other data to evaluate liquefaction potential and develop the Seismic Hazard Zone Map.

Approximately 95 percent of the quadrangle is covered by alluvial deposits of Quaternary age. These Pleistocene through Holocene surficial deposits are summarized in Table 1.1 and discussed below. The remaining area consists of granitic and metamorphic rocks of the Lovejoy Buttes and nearby hills. The bedrock units are discussed in the Earthquake Induced Landslide portion (Section 2) of this report.

Map Unit	Environment of Deposition	Age
Qsc	modern wash	Latest Holocene
Q7	alluvial fan	Late Holocene
Q6	alluvial fan, colluvial aprons	late Pleistocene and Holocene
Quca	alluvial fan, with secondary carbonate	late Pleistocene and Holocene
Q4, Q5	alluvial fan	late Pleistocene
Q3, Q2	alluvial fan	Pleistocene

Table 1.1. Map units used in the Lovejoy Buttes Quadrangle (after Ponti and Burke, 1980).

The oldest Quaternary units on the map (Q3, Q2) are weakly consolidated, uplifted and moderately to severely dissected Pleistocene alluvial fan deposits. These occur in the southern portion of the quadrangle near Llano. Ponti and Burke (1980) subdivided both units into coarse grained (Q3c, Q2c) and very coarse grained (Q3vc, Q2vc). Soils on these materials are moderately to well developed with well-formed horizons and clay accumulations.

Near the center of the map, are isolated areas of late Pleistocene alluvial fan materials (Q4-5c). Ponti and Burke (1980) describe this unit as unconsolidated, uplifted, and slightly dissected alluvial fan deposits. These coarse materials have moderately developed soils, distinct horizons and clay accumulations.

Southeast of the Lovejoy Buttes occurs a medium-grained unit (Quca) rich in secondary calcium carbonate. Ponti and Burke (1980) assume the parent materials to be equivalent to Q4, Q5, and Q6 sediments (late Pleistocene to Holocene). The unit which can contain up to 50 percent calcium carbonate concretions and platy cemented layers, is considered by Ponti (1980) to have been affected by fluctuating groundwater during late Pleistocene and early Holocene.

The most widespread units, Q6c and Q6m, are unconsolidated medium to coarse-grained sediments representing deposition during late Pleistocene to Holocene. Soils on these alluvial fan and colluvial materials are weakly developed.

Within the southwestern portion of the map, the depositional environment is dominated by deposits of Big Rock Wash and the alluvial fan associated with it. The coarse to very coarse-grained alluvial fan units (Q7vc, Q7c) are unconsolidated and have very weakly developed soils. The deposits within the modern washes (Qsc) are also unconsolidated, but have no soil development.

Finer grained units (Q7m, Q7f) occur at the distal ends of modern washes, near playas, and within small drainages associated with the uplifted areas near Llano. These units are unconsolidated and have very weakly developed soils.

Structural Geology

The Lovejoy Buttes Quadrangle occupies a portion of the Antelope Valley, a wedge-shaped part of the Mojave Desert bounded on the northwest by the Garlock Fault and the Tehachapi Mountains, and on the south by the San Andreas Fault and the Transverse Ranges. Within the Lovejoy Buttes Quadrangle, the Llano fault forms the northern boundary of the uplifted Pleistocene sediments Q2 and Q3 and is mapped as having displaced Holocene Q6 and Q7 sediments (fault not shown on Plate 1.1).

ENGINEERING GEOLOGY

Information on subsurface geology and engineering characteristics of Quaternary deposits was obtained from borehole and trench logs collected from geotechnical reports. For this investigation, about 70 borehole and trench logs were collected from the files of Los Angeles County Public Works Department, Earth Systems, and Leighton and Associates. Lithologic and engineering data from 55 logs were entered into the CGS geotechnical GIS database. The characteristics of the Quaternary map units are generalized in Table 1.2 (see Part II -Liquefaction Susceptibility).

From the borehole logs, the Standard Penetration Tests (SPTs) provide a standardized measure of the penetration resistance of geologic deposits and are commonly used as an index of soil density. This in-field test is formally defined and specified by the American Society for Testing and Materials in test method D1586 (ASTM, 1999). Non-SPT geotechnical sampling "blow counts" are converted to SPT-equivalent values. The actual and converted SPT values are normalized to a common-reference [effective-overburden pressure of one atmosphere (approximately one ton per square foot) and a hammer efficiency of 60 percent using a method described by Seed and Idriss (1982) and Seed and others (1985)].

In addition to the SPTs, the results of other engineering tests (dry density, moisture content, sieve analysis, etc.) are used in the Seed-Idriss Simplified Procedure (Seed and Idriss, 1971) to evaluate liquefaction potential of a site (see Part II - Quantitative Liquefaction Analysis). The results of the liquefaction analysis performed on the geotechnical data were posted onto the cross sections and aided in the overall three-dimensional evaluation of the Quaternary deposits.

GROUND WATER

Ground-water conditions were investigated in the Lovejoy Buttes Quadrangle to evaluate the depth to saturated materials. Saturated conditions reduce the effective normal stress, thereby increasing the likelihood of earthquake-induced liquefaction (Youd, 1973). The evaluation was based on first-encountered water noted in water well records and geotechnical borehole logs. Water depths from well logs known to penetrate confined aquifers were avoided, however recent pumping of measured wells or pumping of nearby wells may have affected some measurements.

CGS uses the highest known (historical) ground-water levels for liquefaction evaluation because water levels during an earthquake cannot be anticipated because of the unpredictable fluctuations caused by natural processes and human activities. A historical high ground-water map differs from most ground-water maps that show the actual water table at a particular year or season. Plate 1.2 depicts a hypothetical ground-water table within alluviated areas of the quadrangle. The potential for liquefaction hazard may exist in areas where depth to ground water is 40 feet or less.

Water use in the Antelope Valley increased markedly after the late 1800's, and as a result, ground-water levels significantly dropped during the first half of the century (Carlson and others, 1998; Carlson and Phillips, 1998; Templin and others, 1995). Many wells show that by the late 1970's water levels began to rise again. To evaluate the highest known water levels, the oldest water records were reviewed. In the vicinity of the Lovejoy Buttes Quadrangle, there are records for wells completed prior to the 1950's [California Department of Water Resources (DWR), 1966]; however, the initial water levels for many of these older wells are unknown. Much of the water data for the quadrangle comes from approximately 150 water wells with measurements from the 1950's and 1960's and geotechnical boreholes from the 1960's to the present. In addition, published regional water elevation maps for the years 1958-1965 (Bloyd, 1967), 1979 (Duell, 1987), and 1996 (Carlson and others, 1998) were compared with historical water-well data compiled for this study (DWR, 1966; DWR, 2002) and a shallow ground-water map prepared for Los Angeles County (Leighton, 1990, plate 3).

For the Lovejoy Buttes Quadrangle, those wells with a long history of measurements show that prior to 1964, ground-water levels had dropped 10 to 60 feet/decade. For many of the other wells in the quadrangle, 1964 is the earliest year on record. To evaluate the historical water depth across the quadrangle, water levels from 1964 (for wells with no water level history prior to 1964) were raised 35 feet to approximate a 1940-1950 historical level. These adjusted values were used in conjunction with other well data and borehole logs to produce the contours for the historically high ground-water map (Plate 1.2).

The Lovejoy Buttes act as a partial ground-water barrier resulting in shallow ground-water conditions to the south of the Buttes during times when regional water table elevation approaches the land surface elevations. Locally, within the Buttes in the vicinity of where there used to be a dry lake bed, the presence of a near surface clay zone has periodically produced a measured head of 10 to 15 feet from a depth of 20 to 30 feet

(Leighton, 1968). Geotechnical drilling during 1968 allowed some water to saturate overlying sediments. Later geotechnical investigations nearby confirm that shallow ground-water conditions periodically occur.

The active flood area of Big Rock Wash is included within Plate 1.2 because it receives periodic surface water. Digital orthophoto quarter-quadrangle images for the Lovejoy Buttes Quadrangle were used to define the limits of modern flooding of Big Rock Wash (see Air Photos in References).

PART II

LIQUEFACTION POTENTIAL

Liquefaction may occur in water-saturated sediment during moderate to great earthquakes. Liquefied sediment loses strength and may fail, causing damage to buildings, bridges, and other structures. Many methods for mapping liquefaction hazard have been proposed. Youd (1991) highlights the principal developments and notes some of the widely used criteria. Youd and Perkins (1978) demonstrate the use of geologic criteria as a qualitative characterization of liquefaction susceptibility and introduce the mapping technique of combining a liquefaction susceptibility map and a liquefaction opportunity map to produce a liquefaction potential map. Liquefaction susceptibility is a function of the capacity of sediment to resist liquefaction. Liquefaction opportunity is a function of the potential seismic ground shaking intensity.

The method applied in this study for evaluating liquefaction potential is similar to that of Tinsley and others (1985). Tinsley and others (1985) applied a combination of the techniques used by Seed and others (1983) and Youd and Perkins (1978) for their mapping of liquefaction hazards in the Los Angeles region. CGS's method combines geotechnical analyses, geologic and hydrologic mapping, and probabilistic earthquake shaking estimates, but follows criteria adopted by the SMGB (DOC, 2000).

LIQUEFACTION SUSCEPTIBILITY

Liquefaction susceptibility reflects the relative resistance of a soil to loss of strength when subjected to ground shaking. Physical properties of soil such as sediment grain-size distribution, compaction, cementation, saturation, and depth govern the degree of resistance to liquefaction. Some of these properties can be correlated to a sediment's geologic age and environment of deposition. With increasing age, relative density may increase through cementation of the particles or compaction caused by the weight of the overlying sediment. Grain-size characteristics of a soil also influence susceptibility to liquefaction. Sand is more susceptible than silt or gravel, although silt of low plasticity is treated as liquefiable in this investigation. Cohesive soils generally are not considered susceptible to liquefaction. Such soils may be vulnerable to strength loss with remolding and represent a hazard that is not addressed in this investigation. Soil characteristics and processes that result in higher measured penetration resistances generally indicate lower liquefaction susceptibility. Thus, blow count and cone penetrometer values are useful indicators of liquefaction susceptibility.

Saturation is required for liquefaction, and the liquefaction susceptibility of a soil varies with the depth to ground water. Very shallow ground water increases the susceptibility to liquefaction (soil is more likely to liquefy). Soils that lack resistance (susceptible soils) typically are saturated, loose and sandy. Soils resistant to liquefaction include all soil types that are dry, cohesive, or sufficiently dense.

CGS's map inventory of areas containing soils susceptible to liquefaction begins with evaluation of geologic maps and historical occurrences, cross-sections, geotechnical test data, geomorphology, and ground-water hydrology. Soil properties and soil conditions such as type, age, texture, color, and consistency, along with historical depths to ground water are used to identify, characterize, and correlate susceptible soils. Because Quaternary geologic mapping is based on similar soil observations, liquefaction susceptibility maps typically are similar to Quaternary geologic maps. CGS's qualitative relations between geologic map unit and susceptibility are summarized in Table 1.2.

Most Holocene materials where water levels are within 30 to 40 feet of the ground surface have susceptibility assignments of high to very high. Although some Holocene units may be fine grained, many may contain lenses of material with higher liquefaction susceptibility. Within the Lovejoy Buttes Quadrangle the Holocene units (Qsc, Qsvc, Q7vc, Q7c, Q6c, and Q6m) are all considered highly susceptible. Where available, borehole and trench logs for these materials generally encountered intervals of clean-sorted sands with intervals of gravelly, silty and clayey sand, silt and clay. Materials were generally loose to medium dense as recorded in both descriptions and blow count data.

Geologic Map Unit	Sediment Type	Consistency	Age	Susceptible to Liquefaction?*
Qs	cobbles, gravel, sand	loose	Latest Holocene	yes
Q7	cobbles, gravel, sand	loose	Late Holocene	yes
Q6	cobbles, gravel, sand, silt, and clay	loose to medium dense	late Pleistocene and Holocene	yes
Quca	gravel, sand, and silt with carbonate	cemented	late Pleistocene and Holocene	not likely
Q4, Q5	gravel, sand, and silt	medium to very dense	Late Pleistocene	not likely
Q3, Q2	gravel, sand and silt	medium to very dense	Pleistocene	no

Table 1.2. Quaternary map units used in the Lovejoy Buttes 7.5-Minute Quadrangle and their geotechnical characteristics and liquefaction susceptibility (*when saturated).

LIQUEFACTION OPPORTUNITY

Liquefaction opportunity is a measure, expressed in probabilistic terms, of the potential for strong ground shaking. Analyses of in-situ liquefaction resistance require assessment of liquefaction opportunity. The minimum level of seismic excitation to be used for such purposes is the level of peak ground acceleration (PGA) with a 10 percent probability of exceedance over a 50-year period (DOC, 2000). The earthquake magnitude used in CGS's analysis is the magnitude that contributes most to the calculated PGA for an area.

For the Lovejoy Buttes Quadrangle, PGAs of 0.39 to 0.69g, resulting from an earthquake of magnitude 7.8, were used for liquefaction analyses. The PGA and magnitude values were based on de-aggregation of the probabilistic hazard at the 10 percent in 50-year hazard level (Petersen and others, 1996; Cramer and Petersen, 1996). See the ground motion portion (Section 3) of this report for further details.

Quantitative Liquefaction Analysis

CGS performs quantitative analysis of geotechnical data to evaluate liquefaction potential using the Seed-Idriss Simplified Procedure (Seed and Idriss, 1971; Seed and others, 1983; National Research Council, 1985; Seed and others, 1985; Seed and Harder, 1990; Youd and Idriss, 1997; Youd and others, 2001). Using the Seed-Idriss Simplified Procedure one can calculate soil resistance to liquefaction, expressed in terms of cyclic resistance ratio (CRR), based on SPT results, ground-water level, soil density, moisture content, soil type, and sample depth. CRR values are then compared to calculated earthquake-generated shear stresses expressed in terms of cyclic stress ratio (CSR).

The Seed-Idriss Simplified Procedure requires normalizing earthquake loading relative to a M7.5 event for the liquefaction analysis. To accomplish this, CGS's analysis uses the Idriss magnitude-scaling factor (MSF) (Youd and Idriss, 1997). It is convenient to think in terms of a factor of safety (FS) relative to liquefaction, where: FS = (CRR / CSR) * MSF. FS, therefore, is a quantitative measure of liquefaction potential. CGS uses a factor of safety of 1.0 or less, where CSR equals or exceeds CRR, to indicate the presence of potentially liquefiable soil. While an FS of 1.0 is considered the "trigger" for liquefaction, for a site specific analysis an FS of as much as 1.5 may be appropriate depending on the vulnerability of the site and related structures.

The CGS liquefaction analysis program calculates an FS for each geotechnical sample where blow counts were collected. Typically, multiple samples are collected for each borehole. The program then independently calculates an FS for each non-clay layer that includes at least one penetration test using the minimum $(N_1)_{60}$ value for that layer. The minimum FS value of the layers penetrated by the borehole is used to determine the liquefaction potential for each borehole location. The reliability of FS values varies according to the quality of the geotechnical data. FS, as well as other considerations such as slope, presence of free faces, and thickness and depth of potentially liquefiable soil, are evaluated in order to construct liquefaction potential maps, which are then used to make a map showing zones of required investigation.

Of the 55 geotechnical borehole logs reviewed in this study (Plate 1.2), 25 include blow-count data from SPTs or from penetration tests that allow reasonable blow count translations to SPT-equivalent values. Non-SPT values, such as those resulting from the use of 2-inch or 2½-inch inside-diameter ring samplers, were translated to SPT-equivalent values if reasonable factors could be used in conversion calculations. The reliability of the SPT-equivalent values varies. Therefore, they are weighted and used in a more qualitative manner. Few borehole logs, however, include all of the information (e.g. soil density, moisture content, sieve analysis, etc.) required for an ideal Seed-Idriss Simplified Procedure. For boreholes having acceptable penetration tests, liquefaction analysis is performed using recorded density, moisture, and sieve test values or using averaged test values of similar materials.

The Seed-Idriss Simplified Procedure for liquefaction evaluation was developed primarily for clean sand and silty sand. As described above, results depend greatly on accurate evaluation of in-situ soil density as measured by the number of soil penetration blow counts using an SPT sampler. However, many of the Holocene alluvial deposits in the study area contain a significant amount of gravel. In the past, gravelly soils were considered not to be susceptible to liquefaction because the high permeability of these soils presumably would allow the dissipation of pore pressures before liquefaction could occur. However, liquefaction in gravelly soils has been observed during earthquakes, and recent laboratory studies have shown that gravelly soils are susceptible to liquefaction (Ishihara, 1985; Harder and Seed, 1986; Budiman and Mohammadi, 1995; Evans and Zhou, 1995; and Sy and others, 1995). SPT-derived density measurements in gravelly soils are unreliable and generally too high. They are likely to lead to overestimation of the density of the soil and, therefore, result in an underestimation of the liquefaction susceptibility. To identify potentially liquefiable units where the N values appear to have been affected by gravel content, correlations were made with boreholes in the same unit where the N values do not appear to have been affected by gravel content.

LIQUEFACTION ZONES

Criteria for Zoning

Areas underlain by materials susceptible to liquefaction during an earthquake were included in liquefaction zones using criteria developed by the Seismic Hazards Mapping Act Advisory Committee and adopted by the SMGB (DOC, 2000). Under those guideline criteria, liquefaction zones are areas meeting one or more of the following:

1. Areas known to have experienced liquefaction during historical earthquakes

2. All areas of uncompacted artificial fill containing liquefaction-susceptible material that are saturated, nearly saturated, or may be expected to become saturated

3. Areas where sufficient existing geotechnical data and analyses indicate that the soils are potentially liquefiable

4. Areas where existing geotechnical data are insufficient

In areas of limited or no geotechnical data, susceptibility zones may be identified by geologic criteria as follows:

a) Areas containing soil deposits of late Holocene age (current river channels and their historic floodplains, marshes and estuaries), where the M7.5-weighted peak acceleration that has a 10 percent probability of being exceeded in 50 years is greater than or equal to 0.10 g and the water table is less than 40 feet below the ground surface; or

b) Areas containing soil deposits of Holocene age (less than 11,000 years), where the M7.5-weighted peak acceleration that has a 10 percent probability of being exceeded in 50 years is greater than or equal to 0.20 g and the historical high water table is less than or equal to 30 feet below the ground surface; or

c) Areas containing soil deposits of latest Pleistocene age (11,000 to 15,000 years), where the M7.5-weighted peak acceleration that has a 10 percent probability of being exceeded in 50 years is greater than or equal to 0.30 g and the historical high water table is less than or equal to 20 feet below the ground surface.

Application of SMGB criteria to liquefaction zoning in the Lovejoy Buttes Quadrangle is summarized below.

Areas of Past Liquefaction

In the Lovejoy Buttes Quadrangle, no areas of documented historical liquefaction are known. Areas showing evidence of paleoseismic liquefaction have not been reported.

Artificial Fills

In the Lovejoy Buttes Quadrangle, there were no areas of artificial fill large enough to show at the scale of mapping.

Areas with Sufficient Existing Geotechnical Data

Borehole logs that include penetration test data and sufficiently detailed lithologic descriptions were used to evaluate liquefaction potential. These areas with sufficient geotechnical data were evaluated for zoning based on the liquefaction potential determined by the Seed-Idriss Simplified Procedure. In the Late Pleistocene-Holocene alluvial deposits (Q6m) that are exposed with in the Lovejoy Buttes near Lovejoy Springs, most of the borehole logs that were analyzed using the Seed-Idriss Simplified Procedure contain sediment layers that may liquefy under the expected earthquake loading. This area containing potentially saturated liquefiable material shown in Table 1.2 is included in the zone.

Areas with Insufficient Existing Geotechnical Data

To the south and southwest of the Lovejoy Buttes, Holocene wash and alluvial fan deposits (Qsc, Qsvc, Q7vc, Q7c, Q6c, Q6m) associated with the Big Rock Wash were lacking in geotechnical data. Where these materials occur within the area where the historically highest ground-water occurrence is considered to be at 40 feet or less they are included within the zone. Liquefaction zonation is based on above criteria 4a. Included within the zone is an area previously identified by Leighton and Associates (1990, plate 4) as potentially liquefiable.

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AIR PHOTOS

Digital Orthophoto Quarter Quadrangle Photos, dated 10-4-95, southwest quarter quadrangle area, Lovejoy Buttes Quadrangle. DOQQs and information concerning them can be obtained at http://www-wmc.wr.usgs.gov/doq/

SECTION 2
EARTHQUAKE-INDUCED LANDSLIDE EVALUATION REPORT

Earthquake-Induced Landslide Zones in the Lovejoy Buttes 7.5-Minute Quadrangle, Los Angeles County, California

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PURPOSE

The Seismic Hazards Mapping Act (the Act) of 1990 (Public Resources Code, Chapter 7.8, Division 2) directs the California Department of Conservation (DOC), Division of Mines and Geology (DMG) [now called California Geological Survey (CGS)] to delineate Seismic Hazard Zones. The purpose of the Act is to reduce the threat to public health and safety and to minimize the loss of life and property by identifying and mitigating seismic hazards. Cities, counties, and state agencies are directed to use seismic hazard zone maps prepared by CGS in their land-use planning and permitting processes. The Act requires that site-specific geotechnical investigations be performed prior to permitting most urban development projects within the hazard zones. Evaluation and mitigation of seismic hazards are to be conducted under guidelines established by the California State Mining and Geology Board (DOC, 1997). The text of this report is on the Internet at http://gmw.consrv.ca.gov/shmp/webdocs/sp117.pdf

Following the release of DMG Special Publication 117 (DOC, 1997), agencies in the Los Angeles metropolitan region sought more definitive guidance in the review of geotechnical investigations addressing landslide hazards. The agencies made their request through the Geotechnical Engineering Group of the Los Angeles Section of the American Society of Civil Engineers (ASCE). This group convened an implementation committee in 1998 under the auspices of the Southern California Earthquake Center (SCEC). The committee, which consisted of practicing geotechnical engineers and engineering geologists, released an overview of the practice of landslide analysis, evaluation, and mitigation techniques (SCEC, 2002). This text is also on the Internet at: http://www.scec.org/

This section of the evaluation report summarizes seismic hazard zone mapping for earthquake-induced landslides in the Lovejoy Buttes 7.5-Minute Quadrangle. Section 1 (addressing liquefaction) and Section 3 (addressing earthquake shaking), complete the report, which is one of a series that summarizes the preparation of seismic hazard zone maps within the state (Smith, 1996). Additional information on seismic hazard zone mapping in California can be accessed on the California Geological Survey's Internet page: http://www.conservation.ca.gov/CGS/index.htm

BACKGROUND

Landslides triggered by earthquakes historically have been a significant cause of earthquake damage. In California, large earthquakes such as the 1971 San Fernando, 1989 Loma Prieta, and 1994 Northridge earthquakes triggered landslides that were responsible for destroying or damaging numerous structures, blocking major transportation corridors, and damaging life-line infrastructure. Areas that are most susceptible to earthquake-induced landslides are steep slopes in poorly cemented or highly fractured rocks, areas underlain by loose, weak soils, and areas on or adjacent to existing landslide deposits. These geologic and terrain conditions exist in many parts of California, including numerous hillside areas that have already been developed or are likely to be developed in the future. The opportunity for strong earthquake ground shaking is high in many parts of California because of the presence of numerous active faults. The combination of these factors constitutes a significant seismic hazard throughout much of California, including the hillside areas of the Lovejoy Buttes Quadrangle.

METHODS SUMMARY

The mapping of earthquake-induced landslide hazard zones presented in this report is based on the best available terrain, geologic, geotechnical, and seismological data. If unavailable or significantly outdated, new forms of these data were compiled or generated specifically for this project. The following were collected or generated for this evaluation:

- Digital terrain data were used to provide an up-to-date representation of slope gradient and slope aspect in the study area

- Geologic mapping was used to provide an accurate representation of the spatial distribution of geologic materials in the study area. In addition, a map of existing landslides, whether triggered by earthquakes or not, was prepared

- Geotechnical laboratory test data were collected and statistically analyzed to quantitatively characterize the strength properties and dynamic slope stability of geologic materials in the study area

- Seismological data in the form of CGS probabilistic shaking maps and catalogs of strong-motion records were used to characterize future earthquake shaking within the mapped area

The data collected for this evaluation were processed into a series of GIS layers using commercially available software. A slope stability analysis was performed using the Newmark method of analysis (Newmark, 1965), resulting in a map of landslide hazard potential. The earthquake-induced landslide hazard zone was derived from the landslide hazard potential map according to criteria developed in a CGS pilot study (McCrink and Real, 1996; McCrink, 2001) and adopted by the State Mining and Geology Board (DOC, 2000).

SCOPE AND LIMITATIONS

The methodology used to make this map is based on earthquake ground-shaking estimates, geologic material-strength characteristics and slope gradient. These data are gathered from a variety of outside sources. Although the selection of data used in this evaluation was rigorous, the quality of the data is variable. The State of California and the Department of Conservation make no representations or warranties regarding the accuracy of the data gathered from outside sources.

Earthquake-induced landslide zone maps are intended to prompt more detailed, site-specific geotechnical investigations as required by the Act. As such, these zone maps identify areas where the potential for earthquake-induced landslides is relatively high. Due to limitations in methodology, it should be noted that these zone maps do not necessarily capture all potential earthquake-induced landslide hazards. Earthquake-induced ground failures that are not addressed by this map include those associated with ridge-top spreading and shattered ridges. It should also be noted that no attempt has been made to map potential run-out areas of triggered landslides. It is possible that such run-out areas may extend beyond the zone boundaries. The potential for ground failure resulting from liquefaction-induced lateral spreading of alluvial materials, considered by some to be a form of landsliding, is not specifically addressed by the earthquake-induced landslide zone or this report. See Section 1, Liquefaction Evaluation Report for the Lovejoy Buttes Quadrangle, for more information on the delineation of liquefaction zones.

The remainder of this report describes in more detail the mapping data and processes used to prepare the earthquake-induced landslide zone map for the Lovejoy Buttes

Quadrangle. The information is presented in two parts. Part I covers physiographic, geologic and engineering geologic conditions in the study area. Part II covers the preparation of landslide hazard potential and landslide zone maps.

PART I

PHYSIOGRAPHY

Study Area Location and Physiography

The Lovejoy Buttes 7.5-Minute Quadrangle covers approximately 62 square miles in the Antelope Valley in northeastern Los Angeles County. It includes part of the city of Lake Los Angeles, the community of Llano, and scattered farming residences within unincorporated Los Angeles County land. The center of the area is about 18 miles east of Palmdale and 45 miles northeast of the Los Angeles Civic Center. Topographically, the area is characterized by typical high desert scrubland and grassland of low local relief except for the cluster of hills in the northwestern quarter called Lovejoy Buttes and three small hills in the northeastern corner. Several braided channels of intermittent streams that collectively belong to Big Rock Wash cut across the western part of the quadrangle. One of the peaks of Lovejoy Buttes reaches 3,251 feet in elevation. The highest point in the quadrangle (above 3,380 feet), however, is in the southeastern corner on the alluvial apron that slopes northward from Holcomb Ridge in the adjacent Valyermo Quadrangle. The lowest point, below 2,610 feet, is north of the buttes on the northern boundary. Access to the region is primarily via State Highway 138 (Pearblossom Highway), Palmdale Boulevard, and 170th Street East. Numerous east-west avenues (lettered) and north-south streets (numbered) provide local access.

Digital Terrain Data

The calculation of slope gradient is an essential part of the evaluation of slope stability under earthquake conditions. An accurate slope gradient calculation begins with an up-to-date map representation of the earth’s surface in the form of a digital topographic map. Within the Lovejoy Buttes Quadrangle, a Level 2 digital elevation model (DEM) was obtained from the USGS (U.S. Geological Survey, 1993). This DEM, prepared from the 7.5-minute quadrangle topographic contours based on 1955 aerial photography, has a 10-meter horizontal resolution and a 7.5-meter vertical accuracy.

A slope map was made from the DEM using a third-order, finite difference, center-weighted algorithm (Horn, 1981). The manner in which the slope map was used to prepare the zone map will be described in subsequent sections of this report.

GEOLOGY

Bedrock and Surficial Geology

Dibblee (1967) mapped the bedrock geology of Antelope Valley and vicinity, which includes the Lovejoy Buttes Quadrangle. Ponti and Burke (1980) mapped the Quaternary geology of eastern Antelope Valley and generalized the exposed crystalline basement rocks on their map. The Ponti and Burke (1980) map was digitized for this study by the Southern California Areal Mapping Project [SCAMP].

Bedrock (map symbol gr-m) exposed in the Lovejoy Buttes Quadrangle consists mostly of pre-Tertiary light-colored, massive, medium-grained quartz monzonite with scattered pegmatite dikes (Dibblee, 1967). One mile south of Avenue O near the eastern boundary a small hill consists of hornblende diorite. Pediment (gr-pediment) surfaces are mapped by Ponti and Burke (1980) on Lovejoy Buttes.

Quaternary surficial deposits cover most of the Lovejoy Buttes Quadrangle (Ponti and Burke, 1980). Aprons of coarse-grained colluvium and slopewash surround the buttes on pediment surfaces. Northeast of the site of Llano along State Highway 138, east of Big Rock Wash, Dibblee (1967) mapped a northwest-trending fault along which older, dissected alluvium has been elevated on the south side. Additional discussion the Quaternary deposits can be found in Section 1.

Structural Geology

The entire quadrangle is underlain by a granitic batholith that extends across the western Mojave Desert (Dibblee, 1967). The most significant structural feature influencing slope stability is the occurrence of widely spaced joints and fractures in the granitic rocks. These discontinuities provide the planes of weakness for slope instability in an otherwise extremely hard and competent rock.

Landslide Inventory

As a part of the geologic data compilation, an inventory of existing landslides in the Lovejoy Buttes Quadrangle was prepared by field reconnaissance, analysis of stereo-paired aerial photographs (see Air Photos in References) and a review of previously published landslide mapping. No landslides were found in the Lovejoy Buttes Quadrangle. However, the aprons of coarse-grained colluvial talus and slope wash indicate that rock falls may be the predominant form of slope failure around the buttes.

ENGINEERING GEOLOGY

Geologic Material Strength

To evaluate the stability of geologic materials under earthquake conditions, the geologic map units described above were ranked and grouped on the basis of their shear strength. Generally, the primary source for shear-strength measurements is geotechnical reports prepared by consultants on file with local government permitting departments. No shear tests were found for the Lovejoy Buttes Quadrangle. Shear tests used to characterize geologic units in the Lovejoy Buttes Quadrangle were borrowed from the Hi Vista (3 Qal), Juniper Hills (5 colluvium and slopewash, 1 granitic, 3 Qal), Littlerock (24 Qal), and Palmdale (4 colluvium and slopewash, 1 granitic) quadrangles.

The geologic units of the Lovejoy Buttes Quadrangle were evaluated in three groups. All shear tests of Quaternary units were evaluated as one group, Qal. The other two groups are hard rock (gr) and colluvium on pediment surfaces (gr-pediment). Average (mean or median) phi values for each strength group are summarized in Table 2.1. For the geologic strength groups (Table 2.2) in the map area, a single shear strength value was assigned and used in our slope stability analysis. A geologic material strength map was made based on the groupings presented in Table 2.1 and Table 2.2, and this map provides a spatial representation of material strength for use in the slope stability analysis.

LOVEJOY BUTTES QUADRANGLE SHEAR STRENGTH GROUPS

	Formation Name	Number of Tests	Mean/Median Phi (degrees)	Mean/Median Group Phi (degrees)	Mean/Median Group C (psf)	Phi Values Used in Stability Analysis
GROUP 1	gr	2	42	42	350	42
GROUP 2	gr-pediment/colluvium	9	34/32	34/32	234/203	32
GROUP 3	Qal	30	28	28	185/143	28

Table 2.1. Summary of the Shear Strength Statistics for the Lovejoy Buttes Quadrangle.

SHEAR STRENGTH GROUPS FOR THE LOVEJOY BUTTES 7.5-MINUTE QUADRANGLE		
GROUP 1	**GROUP 2**	**GROUP 3**
gr	gr-pediment/colluvium	Qal

Table 2.2. Summary of Shear Strength Groups for the Lovejoy Buttes Quadrangle.

PART II

EARTHQUAKE-INDUCED LANDSLIDE HAZARD POTENTIAL

Design Strong-Motion Record

To evaluate earthquake-induced landslide hazard potential in the study area, a method of dynamic slope stability analysis developed by Newmark (1965) was used. The Newmark method analyzes dynamic slope stability by calculating the cumulative down-slope displacement for a given earthquake strong-motion time history. As implemented for the preparation of earthquake-induced landslide zones, the Newmark method necessitates the selection of a design earthquake strong-motion record to provide the "ground shaking opportunity". For the Lovejoy Buttes Quadrangle, selection of a strong motion record was based on an estimation of probabilistic ground motion parameters for modal magnitude, modal distance, and peak ground acceleration (PGA). The parameters were estimated from maps prepared by DMG for a 10 percent probability of being exceeded in 50 years (Petersen and others, 1996). The parameters used in the record selection are:

Modal Magnitude:	7.8
Modal Distance:	4.6 to 22.4 km
PGA:	0.36g to 0.74g

The strong-motion record selected for the slope stability analysis in the Lovejoy Buttes Quadrangle was the Southern California Edison (SCE) Lucerne record from the 1992 magnitude 7.3 Landers, California, earthquake. This record had a source to recording site distance of 1.1 km and a peak ground acceleration (PGA) of 0.80g. Although the magnitude and PGA values of the Lucerne record do not fall within the range of the probabilistic parameters, this record was considered to be sufficiently conservative to be used in the stability analyses. The selected strong-motion record was not scaled or otherwise modified prior to its use in the analysis.

Displacement Calculation

The design strong-motion record was used to develop a relationship between landslide displacement and yield acceleration (a_y), defined as the earthquake horizontal ground acceleration above which landslide displacements take place. This relationship was prepared by integrating the design strong-motion record twice for a given acceleration value to find the corresponding displacement, and the process was repeated for a range of acceleration values (Jibson, 1993). The resulting curve in Figure 2.1 represents the full spectrum of displacements that can be expected for the design strong-motion record. This curve provides the required link between anticipated earthquake shaking and estimates of displacement for different combinations of geologic materials and slope gradient, as described in the Slope Stability Analysis section below.

The amount of displacement predicted by the Newmark analysis provides an indication of the relative amount of damage that could be caused by earthquake-induced landsliding. Displacements of 30, 15 and 5 cm are used as criteria for rating levels of earthquake-induced landslide hazard potential based on the work of Youd (1980), Wilson and Keefer (1983), and the CGS pilot study for earthquake-induced landslides (McCrink and Real, 1996; McCrink, 2001). Applied to the curve in Figure 2.1, these displacements correspond to yield accelerations of 0.14, 0.18, and 0.24 g. Because these yield acceleration values are derived from the design strong-motion record, they represent the ground shaking opportunity thresholds that are significant to the Lovejoy Buttes Quadrangle.

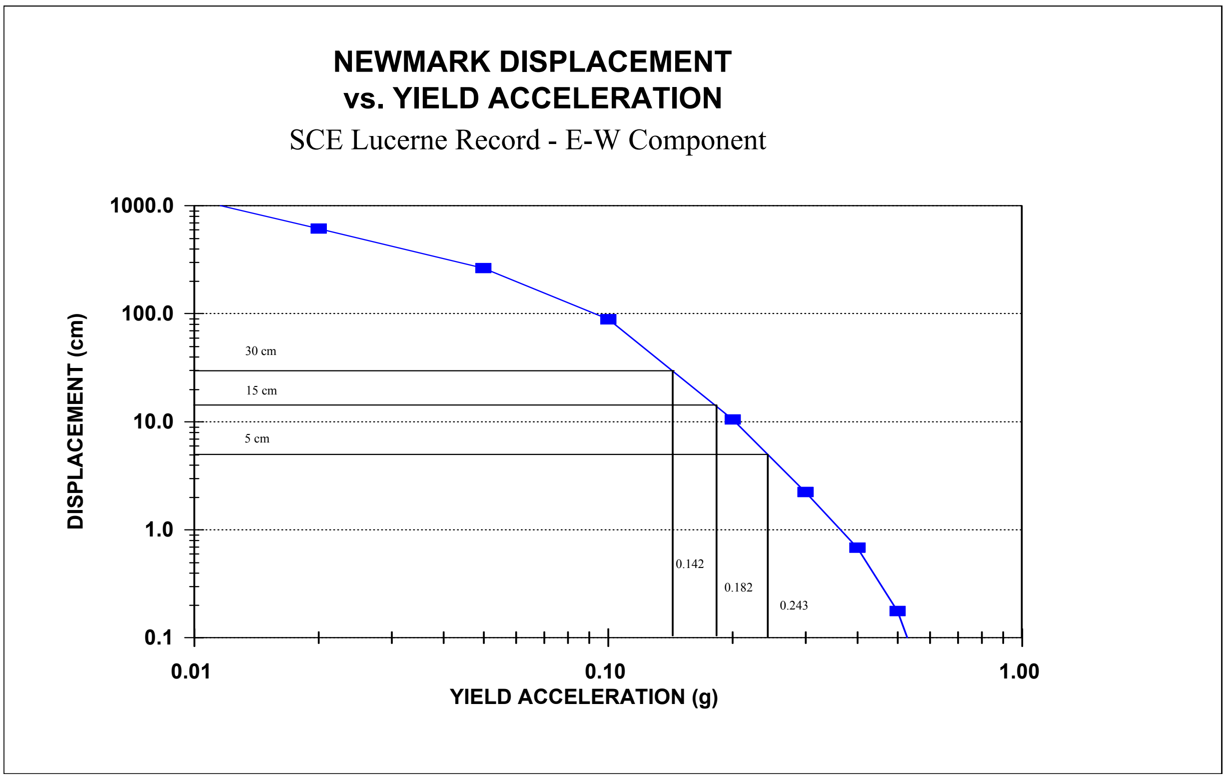

Figure 2.1. Yield Acceleration vs. Newmark Displacement for the 1992 Landers Earthquake SCE Lucerne Record.

Slope Stability Analysis

A slope stability analysis was performed for each geologic material strength group at slope increments of 1 degree. An infinite-slope failure model under unsaturated slope conditions was assumed. A factor of safety was calculated first, followed by calculation of yield acceleration from Newmark's equation:

$$a_y = (FS - 1)g \sin \alpha$$

where FS is the Factor of Safety, g is the acceleration due to gravity, and α is the direction of movement of the slide mass, in degrees measured from the horizontal, when displacement is initiated (Newmark, 1965). For an infinite slope failure, α is the same as the slope angle.

The yield accelerations resulting from Newmark's equations represent the susceptibility to earthquake-induced failure of each geologic material strength group for a range of slope gradients. Based on the relationship between yield acceleration and Newmark displacement shown in Figure 2.1 hazard potentials were assigned as follows:

1. If the calculated yield acceleration was less than 0.14g, Newmark displacement greater than 30 cm is indicated, and a HIGH hazard potential was assigned.

2. Likewise, if the calculated yield acceleration fell between 0.14g and 018.g, Newmark displacement between 15 cm and 30 cm is indicated, and a MODERATE hazard potential was assigned.

3. If the calculated yield acceleration fell between 0.18g and 0.24g, Newmark displacement between 5 cm and 15 cm is indicated, and a LOW hazard potential was assigned.

4. If the calculated yield acceleration was greater than 0.24g, Newmark displacement of less than 5 cm is indicated, and a VERY LOW potential was assigned.

Table 2.3 summarizes the results of the stability analyses. The earthquake-induced landslide hazard potential map was prepared by combining the geologic material-strength map and the slope map according to this table.

LOVEJOY BUTTES QUADRANGLE HAZARD POTENTIAL MATRIX				
Geologic Material Strength Group (Average Phi)	HAZARD POTENTIAL (Percent Slope)			
	Very Low	Low	Moderate	High
1 (42)	0 to 62%	62 to 70%	70 to 72%	>72%
2 (32)	0 to 38%	38 to 44%	44 to 48%	>48%
3 (28)	0 to 30%	30 to 34%	34 to 38%	>38%

Table 2.3. Hazard Potential Matrix for Earthquake-Induced Landslides in the Lovejoy Buttes Quadrangle. Values in the table show the range of slope gradient (expressed as percent slope) corresponding to calculated Newmark displacement ranges from the design earthquake for each material strength group.

EARTHQUAKE-INDUCED LANDSLIDE HAZARD ZONE

Criteria for Zoning

Earthquake-induced landslide zones were delineated using criteria adopted by the California State Mining and Geology Board (DOC, 2000). Under these criteria, earthquake-induced landslide hazard zones are defined as areas that meet one or both of the following conditions:

1. Areas that have been identified as having experienced landslide movement in the past, including all mappable landslide deposits and source areas as well as any landslide that is known to have been triggered by historic earthquake activity.

2. Areas where the geologic and geotechnical data and analyses indicate that the earth materials may be susceptible to earthquake-induced slope failure.

These conditions are discussed in further detail in the following sections.

Existing Landslides

As previously mentioned, no existing landslides were identified in the Lovejoy Buttes Quadrangle. However, the presence of colluvial aprons around the steep sides of the buttes indicates that rock fall, possibly caused by earthquake shaking, is an ongoing geologic process around the buttes. The areas most susceptible to rock fall were identified in the geologic and geotechnical analyses, described below.

Geologic and Geotechnical Analysis

Based on the conclusions of a pilot study performed by CGS (McCrink and Real, 1996; McCrink, 2001), it has been concluded that earthquake-induced landslide hazard zones should encompass all areas that have a High, Moderate or Low level of hazard potential (see Table 2.3). This would include all areas where the analyses indicate earthquake displacements of 5 centimeters or greater. Areas with a Very Low hazard potential, indicating less than 5 centimeters displacement, are excluded from the zone.

As summarized in Table 2.3, all areas characterized by the following geologic strength group and slope gradient conditions are included in the earthquake-induced landslide hazard zone:

1. Geologic Strength Group 3 is included for all slopes steeper than 30 percent.

2. Geologic Strength Group 2 is included for all slopes steeper than 38 percent.

3. Geologic Strength Group 1 is included for all slopes steeper than 62 percent.

This results in less than one percent of the quadrangle lying within the earthquake-induced landslide hazard zone for the Lovejoy Buttes Quadrangle.

ACKNOWLEDGMENTS

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APPENDIX A
SOURCE OF ROCK STRENGTH DATA

SOURCE	NUMBER OF TESTS SELECTED
Littlerock Quadrangle	24
Juniper Hills Quadrangle	9
Palmdale Quadrangle	5
Hi Vista Quadrangle	3
Total Number of Shear Tests	41

SECTION 3
GROUND SHAKING EVALUATION REPORT

Potential Ground Shaking in the Lovejoy Buttes 7.5-Minute Quadrangle, Los Angeles County, California

By

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California Geological Survey
***Formerly with CGS, now with U.S. Geological Survey**

PURPOSE

The Seismic Hazards Mapping Act (the Act) of 1990 (Public Resources Code, Chapter 7.8, Division 2) directs the California Department of Conservation (DOC), Division of Mines and Geology (DMG) [now called California Geological Survey (CGS)] to delineate Seismic Hazard Zones. The purpose of the Act is to reduce the threat to public health and safety and to minimize the loss of life and property by identifying and mitigating seismic hazards. Cities, counties, and state agencies are directed to use the Seismic Hazard Zone Maps in their land-use planning and permitting processes. The Act requires that site-specific geotechnical investigations be performed prior to permitting most urban development projects within the hazard zones. Evaluation and mitigation of seismic hazards are to be conducted under guidelines established by the California State Mining and Geology Board (DOC, 1997). The text of this report is on the Internet at http://gmw.consrv.ca.gov/shmp/webdocs/sp117.pdf

This section of the evaluation report summarizes the ground motions used to evaluate liquefaction and earthquake-induced landslide potential for zoning purposes. Included are ground motion and related maps, a brief overview on how these maps were prepared, precautionary notes concerning their use, and related references. The maps provided herein are presented at a scale of approximately 1:150,000 (scale bar provided on maps), and show the full 7.5-minute quadrangle and portions of the adjacent eight quadrangles. They can be used to assist in the specification of earthquake loading conditions ***for the analysis of ground failure*** according to the "Simple Prescribed Parameter Value" method (SPPV) described in the site investigation guidelines (DOC, 1997). Alternatively, they can be used as a basis for comparing levels of ground motion determined by other methods with the statewide standard.

This section and Sections 1 and 2 (addressing liquefaction and earthquake-induced landslide hazards) constitute a report series that summarizes development of seismic hazard zone maps in the state. Additional information on seismic hazard zone mapping in California can be accessed on the California Geological Survey's Internet page: http://www.conservation.ca.gov/CGS/index.htm

EARTHQUAKE HAZARD MODEL

The estimated ground shaking is derived from the statewide probabilistic seismic hazard evaluation released cooperatively by the California Department of Conservation, Division of Mines and Geology [California Geological Survey], and the U.S. Geological Survey (Petersen and others, 1996). That report documents an extensive 3-year effort to obtain consensus within the scientific community regarding fault parameters that characterize the seismic hazard in California. Fault sources included in the model were evaluated for long-term slip rate, maximum earthquake magnitude, and rupture geometry. These fault parameters, along with historical seismicity, were used to estimate return times of moderate to large earthquakes that contribute to the hazard.

The ground shaking levels are estimated for each of the sources included in the seismic source model using attenuation relations that relate earthquake shaking with magnitude, distance from the earthquake, and type of fault rupture (strike-slip, reverse, normal, or subduction). The published hazard evaluation of Petersen and others (1996) only considers uniform firm-rock site conditions. In this report, however, we extend the hazard analysis to include the hazard of exceeding peak horizontal ground acceleration (PGA) at 10 percent probability of exceedance in 50 years on spatially uniform conditions of rock, soft rock, and alluvium. These soil and rock conditions approximately correspond to site categories defined in Chapter 16 of the Uniform Building Code (ICBO, 1997), which are commonly found in California. We use the attenuation relations of Boore and others (1997), Campbell (1997), Sadigh and others (1997), and Youngs and others (1997) to calculate the ground motions.

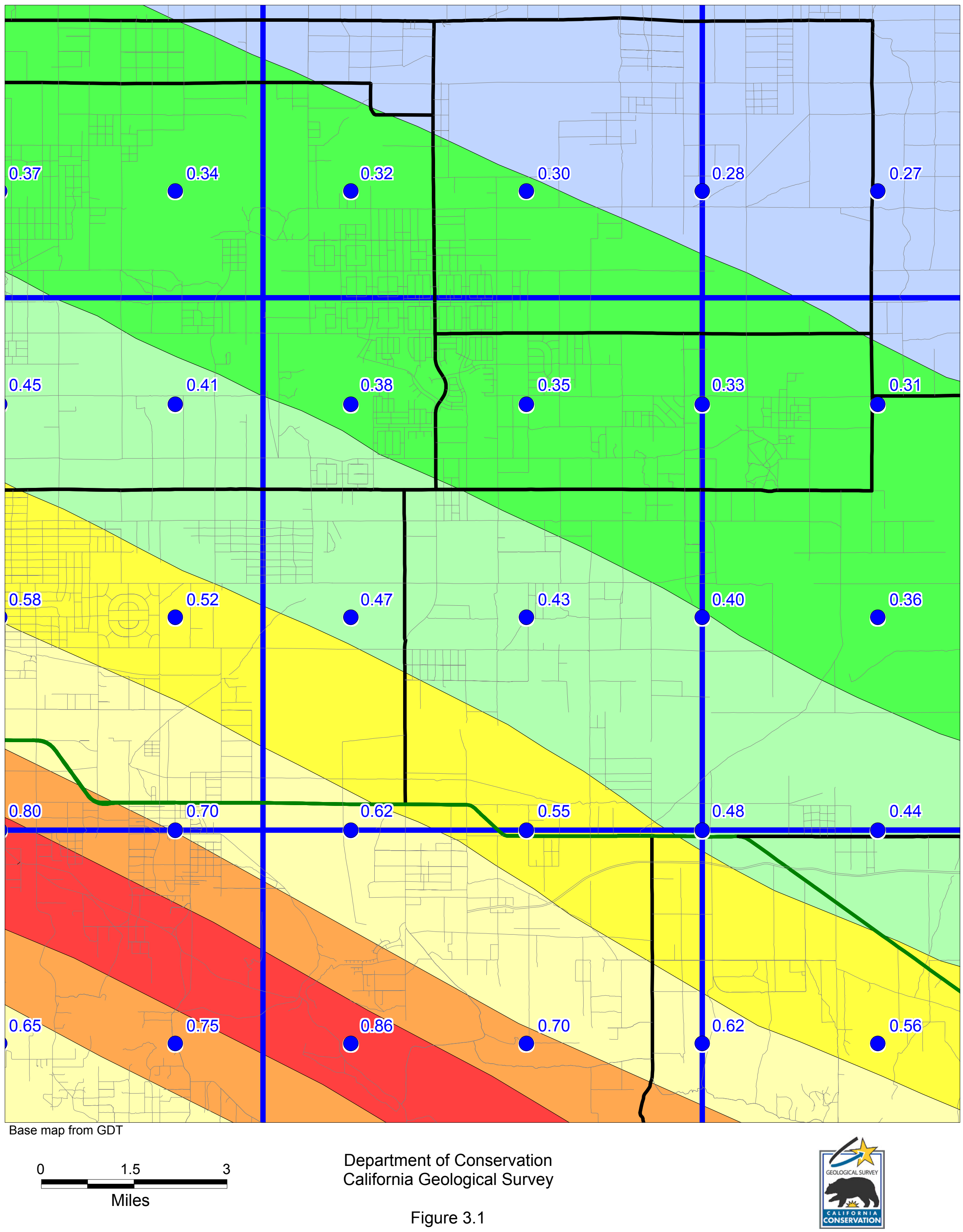
The seismic hazard maps for ground shaking are produced by calculating the hazard at sites separated by about 5 km. Figures 3.1 through 3.3 show the hazard for PGA at 10 percent probability of exceedance in 50 years assuming the entire map area is firm rock, soft rock, or alluvial site conditions respectively. The sites where the hazard is calculated are represented as dots and ground motion contours as shaded regions. The quadrangle of interest is outlined by bold lines and centered on the map. Portions of the eight

LOVEJOY BUTTES 7.5 MINUTE QUADRANGLE AND PORTIONS OF ADJACENT QUADRANGLES

10% EXCEEDANCE IN 50 YEARS PEAK GROUND ACCELERATION (g)

1998

FIRM ROCK CONDITIONS



Department of Conservation
California Geological Survey

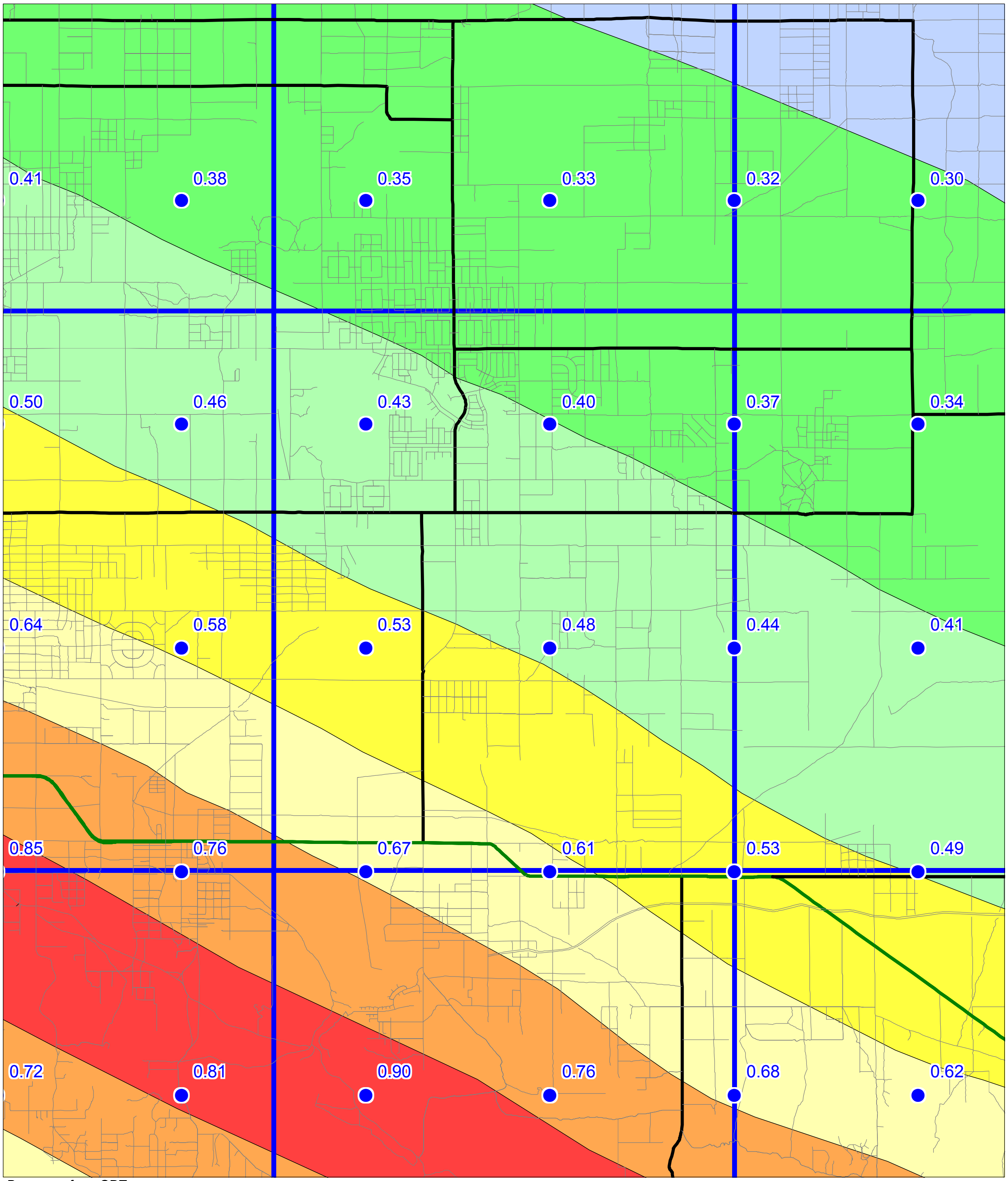
Figure 3.1

LOVEJOY BUTTES 7.5 MINUTE QUADRANGLE AND PORTIONS OF ADJACENT QUADRANGLES

10% EXCEEDANCE IN 50 YEARS PEAK GROUND ACCELERATION (g)

1998

SOFT ROCK CONDITIONS



Base map from GDT

0 1.5 3
Miles

Department of Conservation
California Geological Survey

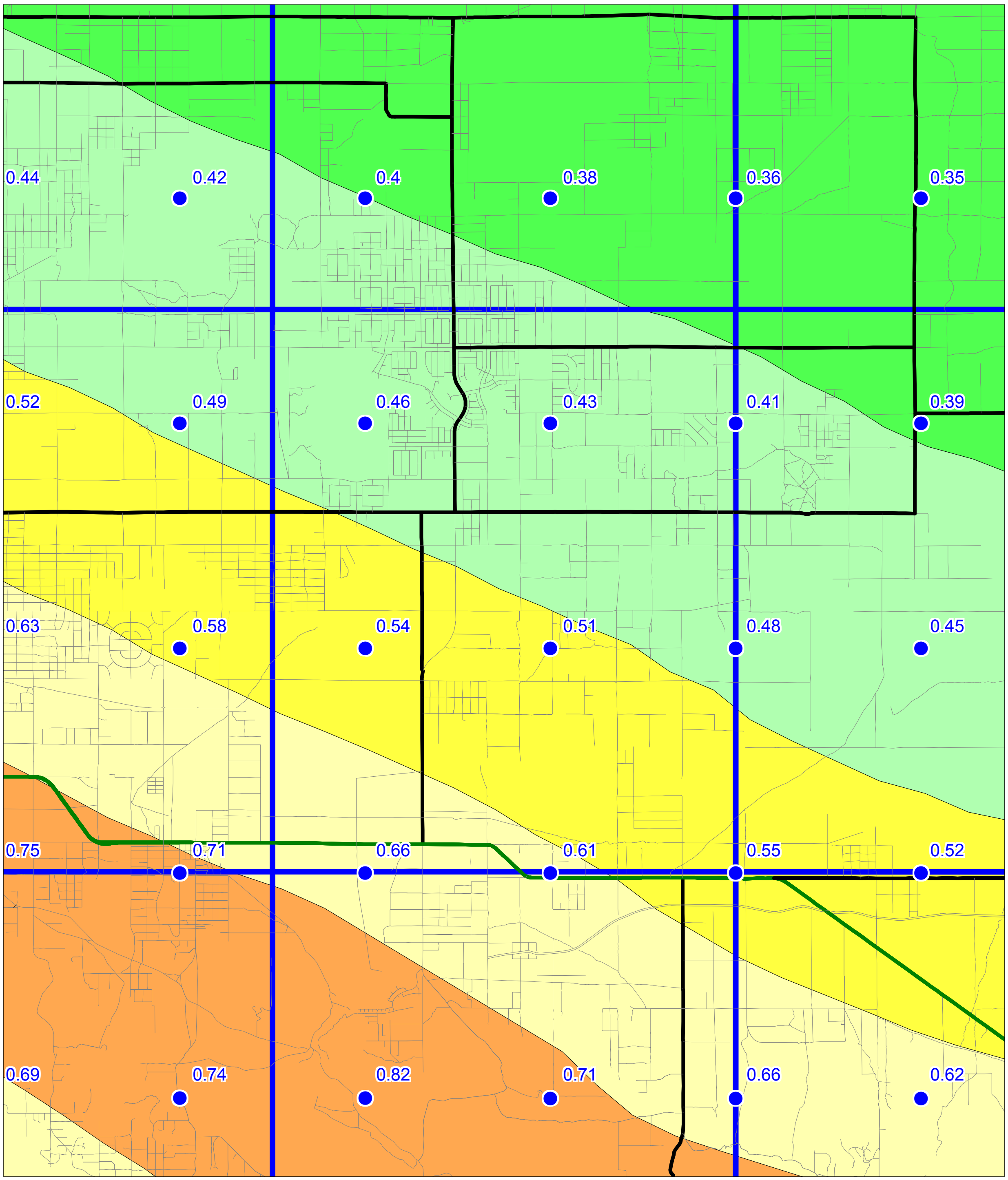
Figure 3.2

LOVEJOY BUTTES 7.5 MINUTE QUADRANGLE AND PORTIONS OF ADJACENT QUADRANGLES

10% EXCEEDANCE IN 50 YEARS PEAK GROUND ACCELERATION (g)

1998

ALLUVIUM CONDITIONS



Base map from GDT

0 1.5 3
Miles

Department of Conservation
California Geological Survey

Figure 3.3

adjacent quadrangles are also shown so that the trends in the ground motion may be more apparent. We recommend estimating ground motion values by selecting the map that matches the actual site conditions, and interpolating from the calculated values of PGA rather than the contours, since the points are more accurate.

APPLICATIONS FOR LIQUEFACTION AND LANDSLIDE HAZARD ASSESSMENTS

Deaggregation of the seismic hazard identifies the contribution of each of the earthquakes (various magnitudes and distances) in the model to the ground motion hazard for a particular exposure period (see Cramer and Petersen, 1996). The map in Figure 3.4 identifies the magnitude and the distance (value in parentheses) of the earthquake that contributes most to the hazard at 10 percent probability of exceedance in 50 years on alluvial site conditions (*predominant earthquake*). This information gives a rationale for selecting a seismic record or ground motion level in evaluating ground failure. However, it is important to keep in mind that more than one earthquake may contribute significantly to the hazard at a site, and those events can have markedly different magnitudes and distances. For liquefaction hazard the predominant earthquake magnitude from Figure 3.4 and PGA from Figure 3.3 (alluvium conditions) can be used with the Youd and Idriss (1997) approach to estimate cyclic stress ratio demand. For landslide hazard the predominant earthquake magnitude and distance can be used to select a seismic record that is consistent with the hazard for calculating the Newmark displacement (Wilson and Keefer, 1983). When selecting the predominant earthquake magnitude and distance, it is advisable to consider the range of values in the vicinity of the site and perform the ground failure analysis accordingly. This would yield a range in ground failure hazard from which recommendations appropriate to the specific project can be made. Grid values for predominant earthquake magnitude and distance should **not** be interpolated at the site location, because these parameters are not continuous functions.

A preferred method of using the probabilistic seismic hazard model and the “simplified Seed-Idriss method” of assessing liquefaction hazard is to apply magnitude scaling probabilistically while calculating peak ground acceleration for alluvium. The result is a “magnitude-weighted” ground motion (liquefaction opportunity) map that can be used directly in the calculation of the cyclic stress ratio threshold for liquefaction and for estimating the factor of safety against liquefaction (Youd and Idriss, 1997). This can provide a better estimate of liquefaction hazard than use of predominate magnitude described above, because all magnitudes contributing to the estimate are used to weight the probabilistic calculation of peak ground acceleration (Real and others, 2000). Thus, large distant earthquakes that occur less frequently but contribute *more* to the liquefaction hazard are appropriately accounted for.

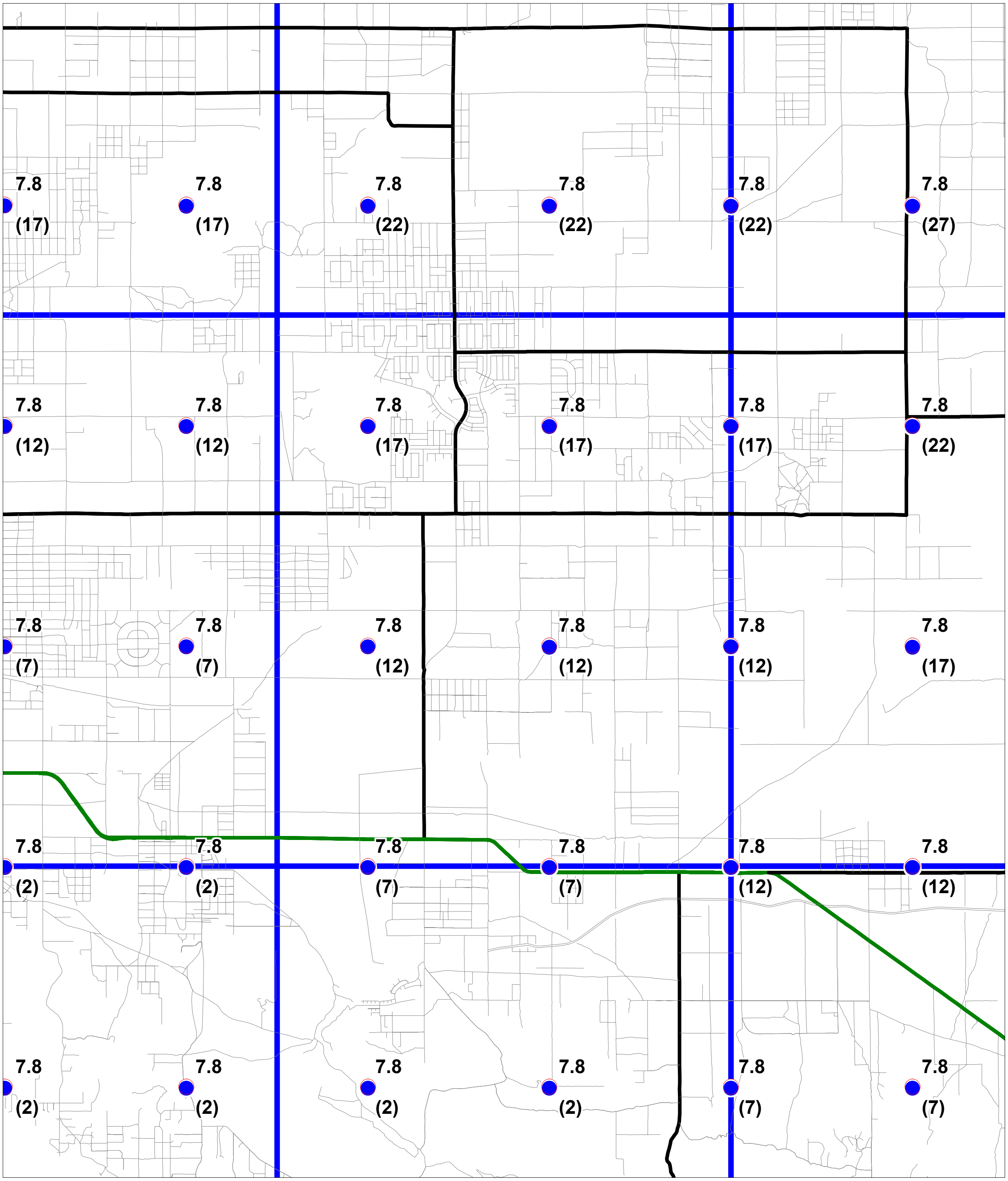
Figure 3.5 shows the magnitude-weighted alluvial PGA based on Idriss’ weighting function (Youd and Idriss, 1997). It is important to note that the values obtained from this map are pseudo-accelerations and should be used in the formula for factor of safety without any magnitude-scaling (a factor of 1) applied.

LOVEJOY BUTTES 7.5 MINUTE QUADRANGLE AND PORTIONS OF ADJACENT QUADRANGLES

10% EXCEEDANCE IN 50 YEARS PEAK GROUND ACCELERATION

1998

PREDOMINANT EARTHQUAKE
Magnitude (Mw)
(Distance (km))



Base map from GDT

0 1.5 3
Miles

Department of Conservation
California Geological Survey

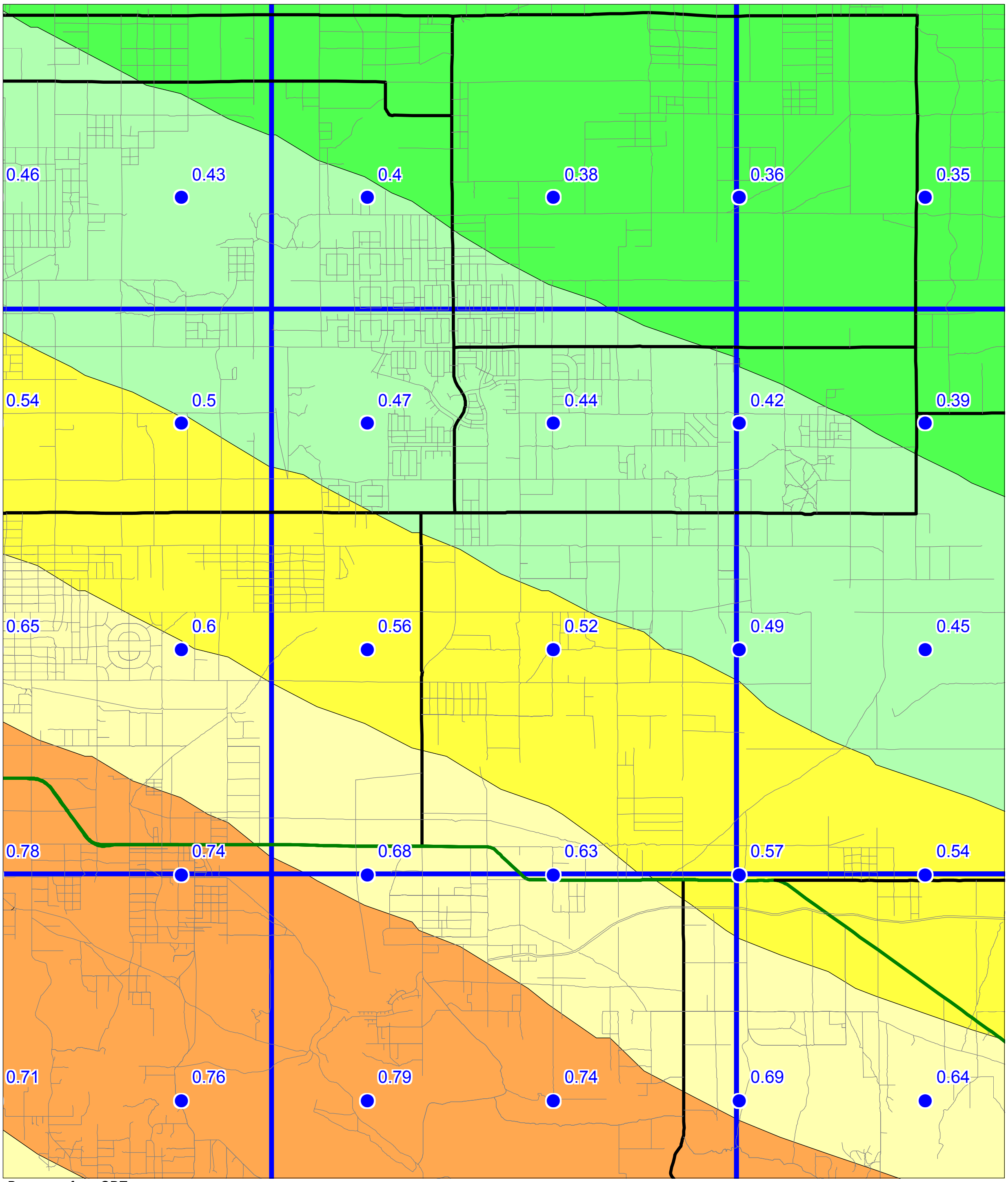
Figure 3.4

LOVEJOY BUTTES 7.5 MINUTE QUADRANGLE AND PORTIONS OF ADJACENT QUADRANGLES

10% EXCEEDANCE IN 50 YEARS MAGNITUDE-WEIGHTED PSEUDO-PEAK ACCELERATION (g) FOR ALLUVIUM

1998

LIQUEFACTION OPPORTUNITY



Base map from GDT

0 1.5 3
Miles

Department of Conservation
California Geological Survey

Figure 3.5

USE AND LIMITATIONS

The statewide map of seismic hazard has been developed using regional information and is ***not appropriate for site specific structural design applications***. Use of the ground motion maps prepared at larger scale is limited to estimating earthquake loading conditions for preliminary assessment of ground failure at a specific location. We recommend consideration of site-specific analyses before deciding on the sole use of these maps for several reasons.

1. The seismogenic sources used to generate the peak ground accelerations were digitized from the 1:750,000-scale fault activity map of Jennings (1994). Uncertainties in fault location are estimated to be about 1 to 2 kilometers (Petersen and others, 1996). Therefore, differences in the location of calculated hazard values may also differ by a similar amount. At a specific location, however, the log-linear attenuation of ground motion with distance renders hazard estimates less sensitive to uncertainties in source location.

2. The hazard was calculated on a grid at sites separated by about 5 km (0.05 degrees). Therefore, the calculated hazard may be located a couple kilometers away from the site. We have provided shaded contours on the maps to indicate regional trends of the hazard model. However, the contours only show regional trends that may not be apparent from points on a single map. Differences of up to 2 km have been observed between contours and individual ground acceleration values. *We recommend that the user interpolate PGA between the grid point values rather than simply using the shaded contours*.

3. Uncertainties in the hazard values have been estimated to be about +/- 50 percent of the ground motion value at two standard deviations (Cramer and others, 1996).

4. Not all active faults in California are included in this model. For example, faults that do not have documented slip rates are not included in the source model. Scientific research may identify active faults that have not been previously recognized. Therefore, future versions of the hazard model may include other faults and omit faults that are currently considered.

5. A map of the predominant earthquake magnitude and distance is provided from the deaggregation of the probabilistic seismic hazard model. However, it is important to recognize that a site may have more than one earthquake that contributes significantly to the hazard. Therefore, in some cases earthquakes other than the predominant earthquake should also be considered.

Because of its simplicity, it is likely that the SPPV method (DOC, 1997) will be widely used to estimate earthquake shaking loading conditions for the evaluation of ground failure hazards. It should be kept in mind that ground motions at a given distance from an earthquake will vary depending on site-specific characteristics such as geology, soil properties, and topography, which may not have been adequately accounted for in the regional hazard analysis. Although this variance is represented to some degree by the recorded ground motions that form the basis of the hazard model used to produce Figures 3.1, 3.2, and 3.3, extreme deviations can occur. More sophisticated methods that take into account other factors that may be present at the site (site amplification, basin effects, near source effects, etc.) should be employed as warranted. The decision to use the SPPV method with ground motions derived from Figures 3.1, 3.2, or 3.3 should be based on careful consideration of the above limitations, the geotechnical and seismological aspects of the project setting, and the "importance" or sensitivity of the proposed building with regard to occupant safety.

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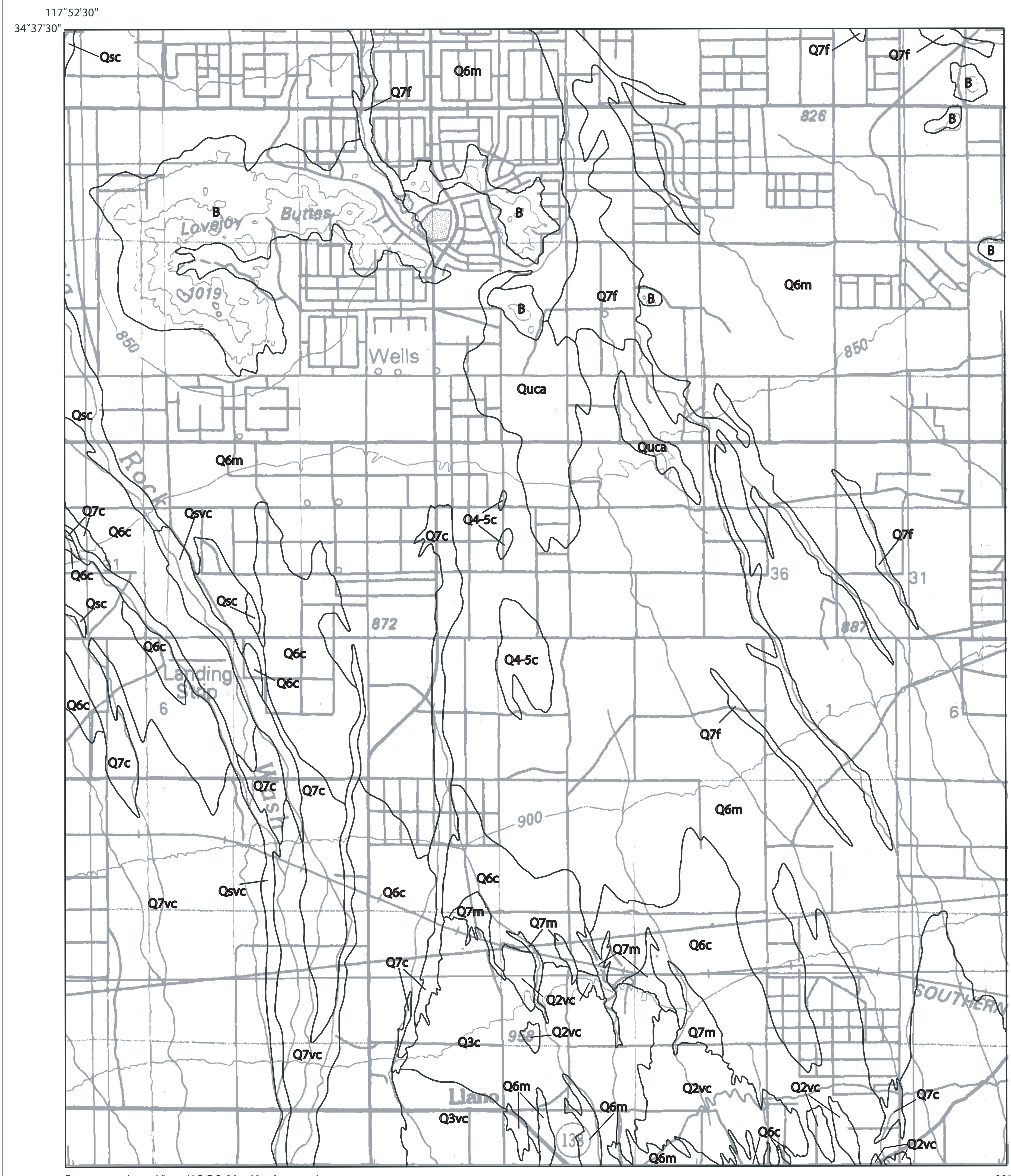


Base map enlarged from U.S.G.S. 30 x 60-minute series

LOVEJOY BUTTES QUADRANGLE

SCALE

1 .5 0 1 MILE
1000 0 1000 2000 3000 4000 5000 6000 7000 FEET
1 .5 0 1 KILOMETER

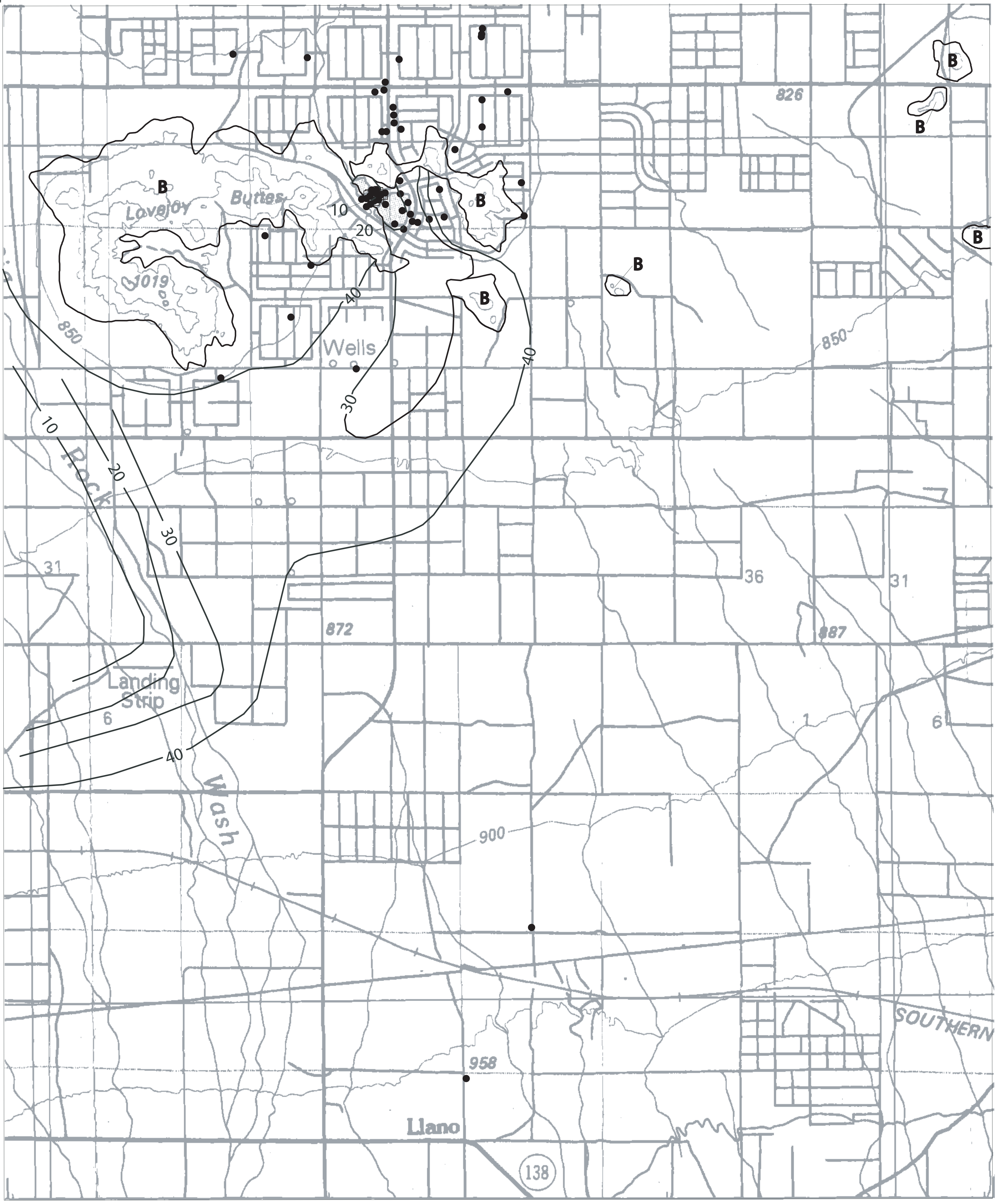
N

B = Pre-Quaternary bedrock.

See "Bedrock and Surficial Geology" in Section 1 of report for descriptions of units.

Plate 1.1 Quaternary Geologic Map of the Lovejoy Buttes 7.5-Minute Quadrangle, California.

117°52'30"
34°37'30"



34°30'
117°45'

Base map enlarged from U.S.G.S. 30 x 60-minute series

LOVEJOY BUTTES QUADRANGLE

SCALE
1 .5 0 1 MILE
1000 0 1000 2000 3000 4000 5000 6000 7000 FEET
1 .5 0 1 KILOMETER

N

40 Depth to ground water, in feet

B Pre-Quaternary bedrock.
See "Bedrock and Surficial Geology"
in Section 1 of report for descriptions of units.

● Geotechnical borings used in liquefaction evaluation

Plate 1.2 Depth to historically high ground water, and locations of boreholes used in this study, Lovejoy Buttes 7.5-minute Quadrangle, California